

# EVALUATION OF DELAY CHARACTERISTICS AT SIGNALIZED INTERSECTIONS FOR IMPROVEMENT IN LEVEL OF SERVICE

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**Abstract:** In our country traffic flow is heterogeneous and is governed by traffic characteristics as driver behavior, road geometry etc. Increase in traffic volume and limited road way width should be promptly addressed by traffic engineers who shall provide solutions which shall be more competent than traditional signalized designs. The recommendations suggested by the traffic engineer for improving signalized intersection shall be less expensive than providing interchanges. Metropolitan cities like Hyderabad are also experienced with delay and congestion due to ineffective traffic signal operation. Non-lane based traffic behavior, varying pavement width and driver characteristics are a few reasons for delay and congestion on our city roads. Improvement in the above context can be achieved through lane prioritization, alternative routes etc. This improvements will certainly reduce the impact on fuel consumption, improves safety and Level of Service at signalized intersection. In this context a study was carried at selected four major intersections (Bowenpally, Fathenagar, Balanagar and Alwyn) in Hyderabad city and was evaluated for existing traffic signal facilities. Parameters such as delay and queue length and Level of Service (LOS) were considered for evaluation. It is observed from simulation results that all the four intersections provide longer delay, larger queue length on major approaches of the intersections. These intersections were improved by providing alternative route for U-turn and left turn traffic movements which is achieved through channelization. Delays, average queue lengths were improved at all the four intersections. This has also resulted in achieving improved level of services at the all signalized intersections.

**Keywords:** delay, average queue length, network performance, level of service.

## 1. Introduction

Hyderabad Metropolitan Development Authority (HMDA) is the sixth largest city in India, with a population of 7.1 million in the year 2014. The city is one of the fastest growing metropolitan cities with a decadal growth rate of 32%. The urban agglomeration of Hyderabad spread over

an area of 650sq. km which comprises Hyderabad, and twelve other municipal entities surrounding it. Hyderabad is located along the banks of the Musi River. Regionally, Hyderabad lies on the convergence of national highways, state highways, rail routes and air routes, which link it's with other metropolitan centres and key centres of importance in the country.

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The city is experiencing rigorous changes in its infrastructure which is due to rapid urbanization, service oriented activities and industrial development. The city also exhibits that vehicular growth is not in proportionate with the available road geometry. Most of the urban roads in HMDA area are static in behaviour during peak hours. This is leading to traffic congestion and in-turn to endless transportation gridlocks which has a direct impact on economy.

### 1.1. Vehicular Growth and Composition

The number of vehicles registering in the city is also nurturing at a rapid pace which is similar to that of population in our city. Over the last decade in Hyderabad two wheelers have increased by 56 percent, four wheelers 27 percent and three wheelers 50.4 percent. The modal share of private vehicle (cars, three wheelers and two wheelers) is about 61percent. This is because in dearth of public transportation in terms of number of buses per commuter, frequency and its coverage at various internal parts of the city; as a reason of this private vehicle ownership has increased drastically.

### 1.2. Traffic Problems in Hyderabad City

Following are the characteristics of traffic problems in the city, specifically in high volume corridors.

- a. Congestion levels have increased which has resulted in reducing average journey speeds to 18 KMPH in central business developed areas of the city (CBD) (Source: City Development Plan, Greater Hyderabad Municipal Cooperation (GHMC))
- b. Delay at signalized intersections, which is due to the absence of lane based traffic system. This is also increase threat to safety of road users.
- c. Indiscipline parking by the vehicle owners at all CBD areas in the city.
- d. There is an increase in vehicular volumes of para - transit vehicle facility. (three wheeler) This increase the maneuverability problem, indiscriminate stoppages so as to serve the passengers / commuters in the existing traffic situation.
- e. Two wheeler mode shares had increased drastically.
- f. Varying carriageway widths creating turbulence in traffic flow.

### 1.3. Need for the Study

The purpose of this study is to develop a process for automating traffic delay which shall improve the existing traffic situations. From the delay parameters Level of Service are computed and are compared with alternatives suggested for traffic movements at all study locations.

### 1.4. Objectives of Present Study

The objective of this paper is to study the improvement in traffic situations at selected signalized intersection which are served by motorized transportation.

- a. To simulate and evaluate traffic parameters of selected study area using microscopic simulation technique.
- b. To propose an alternative routes / priority based on selected study intersections for improving the existing traffic situations and LOS.

## 2. Literature Review

Extensive studies were carried for improving the traffic situation in urban locations. One of the most prominent early works is (Webster, 1958) explained signal time setting characteristics for arriving analytical delay and queuing parameters. (Sadoun, 2003) explained that city traffic light control is a complex, challenging problem and makes difficult to perform analyses with traditional analytical techniques. The results showed an improvement in vehicular delay and minimizing the corresponding queue length at signalized intersections. (Cheng *et al.*, 2005) explains the use of HCM (2000) for estimation of capacity and delay at signalized intersections. (Qi *et al.*, 2007) determined the lengths of left-turn lanes at signalized intersections by considering the possibility of residual queues. (Hellinga and Abdy, 2008) has elaborated that traffic signal timing plans are typically developed on the basis of turning movement traffic and pedestrian volume. In India traffic behavior is heterogeneous characterized by diverse vehicles, lack of lane discipline and results in a very complex behavior (Mathew and Radhakrishnan, 2010). This provides difficulty for every vehicle to be moving in lane based. (Murat *et al.*, 2014) explored the relationship between cyclic time, vehicle queue and vehicular delay importance at signalized intersection. (Yao and Michael Zhang, 2013), (Yao, 2013; Yao, 2014). (Chai and Wong, 2014) analysed the delay of shared lanes at signalized intersections under different proportions of through and right-turn movements and considering the blockage of either of movements. (Nyantakyi *et al.*, 2014) explained signalized intersections are vital nodal points in a transportation network and their efficiency of operation greatly influences the entire network's

performance. The study concluded that changes in phasing plan with geometric improvement would improve upon the intersection's level of service. (Farivar and Tian, 2016) explained the probabilistic delay model for signalized intersections with right-turn channelization lanes considering the possibility of blockage. Many traffic simulation models are used in various combinations to quantify the traffic signals characteristics. However, the findings of traffic simulation models are not widely adopted for improving urban intersection like Hyderabad city. In this connection an attempt is made for improving the traffic situation by providing alternative directions and lane priority at selected intersections of Hyderabad city using microscopic simulation techniques.

## 3. Methodology

The current study has been carried out in 5 stages as listed below:

- Stage 1: Collection of Literature;
- Stage 2: Traffic data collection through primary and secondary sources;
- Stage 3: Development of base model for all the study locations / intersections;
- Stage 4: Evaluation of existing and alternatives direction routes through simulation techniques;
- Stage 5: Results summarization and Conclusions.

Traffic micro-simulation software VISSIM© 6 (PTV, 2014) was used to arrive the base situation of traffic. Following are parameters considered in development of base model and are discussed as follows. (i) Lane Behaviour: The lane changing behavior on urban roads for Indian conditions were used in simulation model; (ii) Traffic Heterogeneity:

The data collected from the site was used in each intersection to obtain the traffic heterogeneity. The heterogeneous traffic was converted into homogeneous traffic stream with the corresponding Passenger Car Unit (PCU) equivalency factors (Chandra, 2008); (iii) Traffic Signal process: Cycle length, green split time, clearance, phasing was designed and used for evaluation; (iv) Traffic Characteristics: Traffic volumes, turning movements and vehicle compositions for each approach were collected in the field and was arrived for peak hour volume; (v) Vehicle Classification: Vehicles were classified as buses, passenger car, two wheelers three wheelers, HCV etc. The base model was developed considering Indian vehicular dimension and was implemented in VISSIM 6; (vi) Vehicular Speeds: The vehicular speeds were arrived as per the field study; Other Vehicular characteristics as maximum acceleration deceleration, power and vehicle weight, pedestrian interference with the traffic flow were not considered in the scope of the study.

The travel time delay was measured considering the average queue length in each approach of intersections. Average control delay which is arrived from (HCM, 2010) does not consider the delay due to acceleration to desired speed. Therefore, the delay arrived from microscopic simulation observations are slightly greater than field estimates of control delay (Tian *et al.*, 2002). The calibration parameters were used at Bowenpally, Intersection which was validated on remaining intersections in the study (Fathenagar, Balanagar and Alwyn intersections). The root mean square error

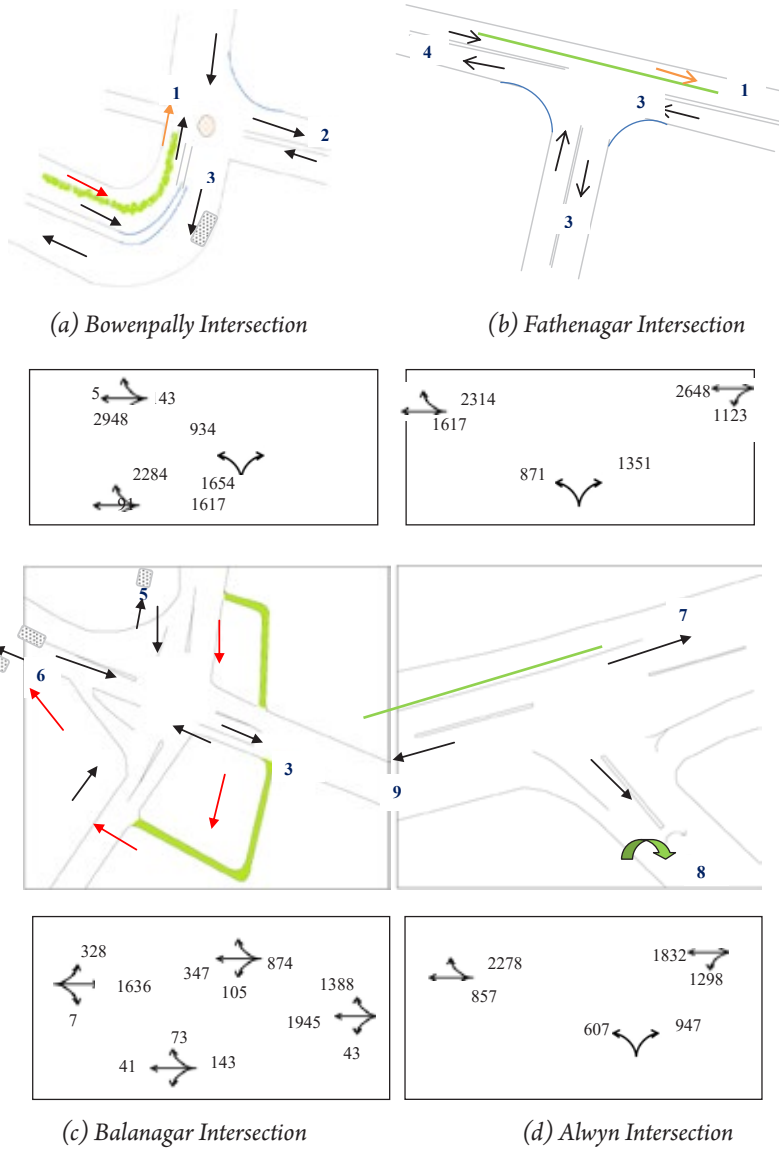
in delay was within the permissible limits and hence confirms for validation.



The developed model was later simulated for both with and without priority, alternative scenarios to appreciate the efficacy in prioritization techniques for the existing traffic situation as shown in Fig. 1. The alternatives demonstrated that the existing roads when allowed for priority traffic routes provides improvement in delay and level of service characteristics than approaches with existing road geometry, on-street parking and improper bus bays.

## 4. Data Collection

### 4.1. Study Area

The study area is located on National Highway (NH – 65) corridor and is passing through Bowenpally, Fathenagar, Balanagar and Alwyn intersection as shown in Fig. 1. Bowenpally is four legged intersection with a provision of bus stop situated at 100m from the intersection. Fathenagar is three legged intersection having on street parking facility on the major approach for three wheeled vehicles. Balanagar is four legged intersection with bus stops situated at 100m on two approaches and with on-street parking facility for three wheeled vehicles. Alwyn intersection is three-legged intersection without on-street parking facilities. All the four intersections are referred as critical due to highest level of interactions of motorized traffic. In this connection the study has suggested for traffic route alternatives, lane priority for straight and ‘U’ turn flow as shown in Fig. 1



 Represents alternative directions, priority lanes for straight and 'U' turn flow at study locations  
 Bus Stop location

**Fig. 1.**

*Geometrical View with Volume and Alternative Route Directions at Study Locations*

1: Towards Bowenpally; 2: Towards Secunderbad; 3: Towards Fathenagar ; 4 : Towards Balanagar; 5: Towards Jeedimetla; 6: Towards Y Junction; 7:Towards :Miyapur; 8: Towards Kondapur; 9: Towards Lingampally

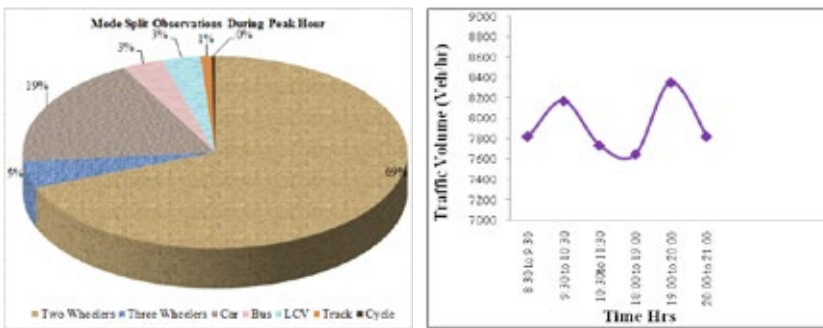
Traffic data was collected at all four intersections during morning and evening periods of week day traffic through video recording technique and is shown in Fig. 1.

and lane priority. Optimum cycle time, phases, minimum green time, clearance times were designed using IRC and was simulated using VISSIM 6.

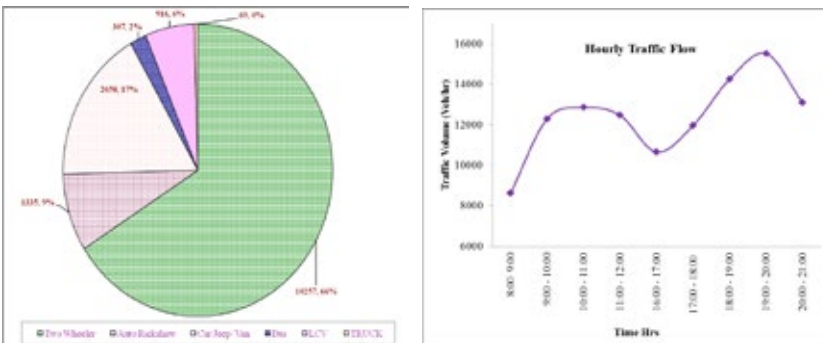
This data is used as input for development of base model and for evaluation of existing situation using microscopic simulation technique. Performance of improvement was measured by considering delay as parameter by providing alternatives route directions

### 5. Results and Discussion

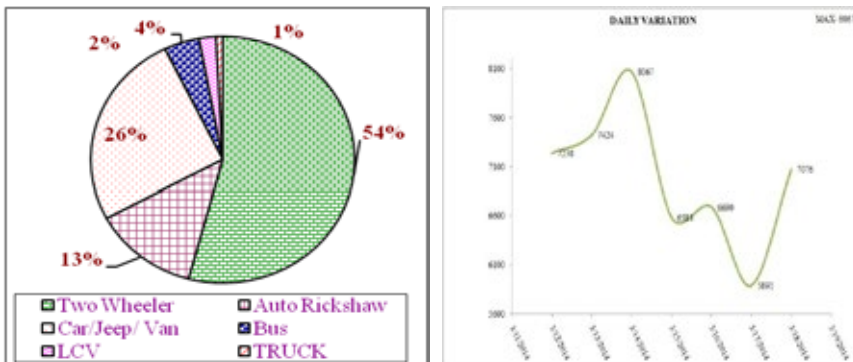
Peak hour volume at all the four intersections was considered for signal design. Mode share observations and peak hour volume is shown in Figs. 2, 3, 4 and 5.



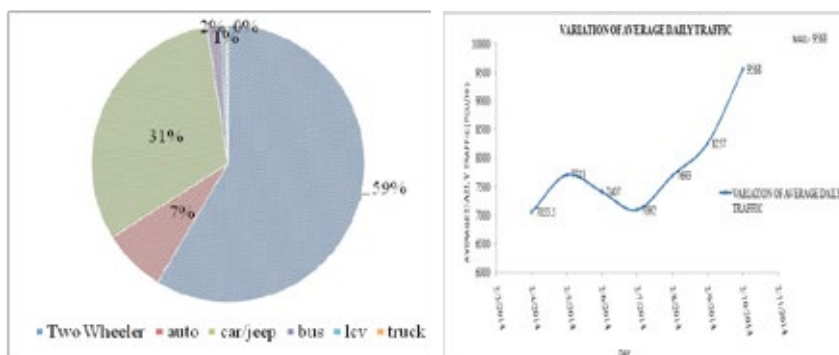
**Fig. 2.** Mode Split and Peak Hour Traffic Flow Variation at Bowenpally Intersection



**Fig. 3.** Mode Split and Peak Hour Traffic Flow Variation Fathenagar Intersection



**Fig. 4.**  
Mode Split and Peak Hour Traffic Flow Variation Balanagar Intersection



**Fig. 5.**  
Mode Split and Peak Hour Traffic Flow Variation Alwyn Intersection

The composition of two wheelers is close to 58 % at all the four intersections, next share composite to passenger car of 30% and remaining to three wheelers, LCV, buses and Truck.

### 5.1. Improvements at Study Locations for Alternatives

#### Bowenpally Intersection

Traffic simulation is carried for Bowenpally Intersection with and without providing priority. It is observed that the delays are reduced from 165.7 sec to 37.9 sec and is shown in Table 1.

**Table 1***Simulation Results without and with Prioritization(Left Lane) At Bowenpally Intersection*

From link	Vehicles Originating from	To link	Vehicles Designating to	Direction of Traffic	Without Improvement		With Improvement	
					Delay (Sec)	Queue length (m)	Delay (Sec)	Queue length (m)
1	Fathenagar	4	Bowenpally	Straight	58.4	42.0	58.4	42
2	Secunderbad	3	Bowenpally	Right Turn	85.6	30.7	25.4	10.0
5	Fathenagar	6	Secunderbad	Right Turn	129.9	109.3	129.9	109.3
7	Bowenpally	8	Fathenagar	Straight	35.0	13.7	35.0	13.7
9	Secunderbad	10	Fathenagar	Left Turn	165.7	46.1	37.9	12.0
11	Bowenpally	11	Secunderbad	Left Turn	27.7	20.0	27.7	20.0

It is observed from the above Table that link 9 – 10 (i.e.) Secunderbad to Fathenagar has the delay of 65.7s due to conflict area between motorists from Secunderbad to Fathenagar and Fathenagar to Secunderabad. This is improved through prioritization of left side traffic movement of Fathenagar to Secunderabad approach.

#### *Fathenagar Intersection*

*Fathenagar Intersection* a three legged ‘T’ intersection. Major approach is running through Bowenpally and Balanagar. Minor approach connects to Fathenagar. Free left

lane is suggested through prioritization and simulated for with and without improvements as in shown Table 2.

It is observed from the Table 2 that the link 8 – 9 (i.e.) Fathenagar to Balanagar has a vehicular delay of 202.5s. This is mainly due reduction of left lane at Fathenagar approach which is occupied by three wheelers parking and is resulting in enormous conflicts between motorists link 9-8. Improvement of this is a result of left traffic lane prioritization from Fathenagar to Balanagar and Balanagar to Bowenpally with improvement in delay as 202.5 s to 30 s and 124 to 12.8 s.

**Table 2***Simulation Results without and with Prioritization at Fathenagar Intersection*

From link	Vehicles Originating from	To link	Vehicles Designating to	Direction of Traffic	Without Improvement		With Improvement	
					Delay (Sec)	Queue length (m)	Delay (Sec)	Queue length (m)
1	Fathenagar	12	Bowenpally	Right turn	52.5	54.9	28.2	54.9
2	Balanagar	3	Bowenpally	Straight	186.5	87.8	0.0	0.0
4	Balanagar	5	Fathenagar	Right turn	118.2	79.0	58.7	79.0
6	Bowenpally	7	Fathenagar	Left turn	55.0	54.3	67.2	87.3
8	Fathenagar	9	Balanagar	Left turn	202.5	90.3	30.0	23.0
10	Bowenpally	11	Balanagar	Straight	124.0	81.3	12.8	35.0

#### *Balanagar Intersection*

*Balanagar Intersection* is a four legged intersection. Major approach runs from Kukatpally to Fathenagar and Minor approach runs to a major approach – Moosapet to Jeedimetla. Simulation is

carried with and without alternative directions at the intersection. Alternative direction is provided for the traffic coming from Fathenagar to Balanagar. Results are tabulated in the Table 3.



**Table 3***Simulation Results without Left Lane Alternative Route at Balanagar Intersection*

From Link	Vehicles Originating from	To Link	Vehicles Designating to	Direction of Traffic	Without Improvement		With Improvement	
					Delay (Sec)	Queue length (m)	Delay (Sec)	Queue length (m)
1	Fathenagar	1	Kukatpally	Straight	120.4	93.9	75.5	83.2
1	Fathenagar	5	Jeedimetla	Right Turn	140.6	93.9	86.7	83.2
1	Fathenagar	7	Moosapet	Free Left	123.8	65.5	10.0	57.9
2	Kukatpally	1	Y Junction	U Turn	110.7	40.3	40.1	40.6
2	Kukatpally	2	Fathenagar	Straight	52.7	40.3	9.0	40.6
2	Kukatpally	7	Moosapet	Right Turn	20.9	40.3	0.0	0.0
4	Jeedimetla	1	Y Junction	Right Turn	128.8	65.3	0.0	13.5
4	Jeedimetla	2	Fathenagar	Free Left	67.1	23.8	26.2	35.6
4	Jeedimetla	7	Moosapet	Straight	106.6	57.9	3.7	0.0
6	Moosapet	1	Kukatpally	Free Left	4.1	10.1	0.0	0.0
6	Moosapet	2	Fathenagar	Right Turn	2812.2	10.1	0.1	0.0

From the simulation results it is observed that by providing alternative left direction route from Jeedimetla to Balanagar and Balanagar to Y Junction delays were reduced. The Average queue lengths are also reduced considerably.

*Alwyn Intersection*

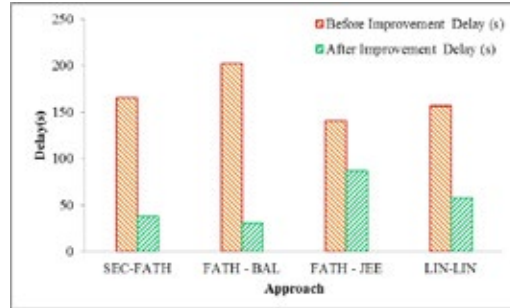
Traffic simulation is carried at Alwyn intersection with and without providing U-turn priority. The simulation results for with and without priority are tabulated in Table 4.

**Table 4***Simulation Results without U-Turn Priority at Alwyn Junction*

From Link	Vehicles Originating from	To Link	Vehicles Designating to	Direction of Traffic	Without Improvement	With Improvement	Without Improvement	With Improvement
					Delay (Sec)	Queue length (m)	Delay (Sec)	Queue length (m)
1	Lingampally	1	Miyapur	Free Left	26.2	34	20.9	19
1	Lingampally	2	Lingampally	U Turn	156.3	71	57.4	21
1	Lingampally	4	Kondapur	Right Turn	68.4	36.3	53.2	40.8
2	Miyapur	1	Miyapur	U Turn	74.9	50.9	68.7	44.3
2	Miyapur	2	Lingampally	Straight	35	50.9	35	44.3
3	Miyapur	4	Kondapur	Left Turn	32.6	0.0	31	0.0
5	Kondapur	1	Miyapur	Right Turn	84.1	35.1	33.9	21.9
6	Kondapur	7	Lingampally	Left Turn	11.2	0.0	12.0	0.0
5	Kondapur	4	Kondapur	U Turn	1.0	0.0	1.0	0.0

From the simulation results it is observed that delay is reduced from 156.3 sec to 57.4 sec for Lingampally to Lingampally approach.

The Average queue length is also reduced considerably. The variation in delay at all intersections (approach legs) is shown in Fig. 6.



**Fig. 6.**  
*Variation in Delay Improvements at All Approaches of Intersections*

**5.2. Effect in Level of Service for Improvements**

Delay parameters are used to estimate Level of Service (LOS). The LOS with and with

prioritizations at all study locations is shown in Table 5. Further improvement in LOS was not possible because of the existing road geometry, land use pattern of commercial and industrial establishment.

**Table 5**  
*Level of Service at the All Four Intersections without and with Improvements*

Name of the Intersection	Without Improvement	With Improvement	Before Improvement	After Improvement
	Aggregate delay (Sec's)	Aggregate delay (Sec's)	LOS	LOS
Bowenpally	83.76	52.1	F	D
Fathenagar	123.11	32.8	F	C
Balanagar	74.623	40.1	E	D
Alwyn	54.11	39.1	D	C

(Source for LOS calculations is per HCM, 2000)

It is inferred that Level of Service for all the intersections has improved. This is achieved because of providing alternative routes, lane priority for left turn's and straight flow movements at study intersections.

**6. Summary and Conclusions**

The study reveals that alternatives is an instrumental tool in reducing vehicular delay, queue length and level of service at urban intersections having heterogeneity road traffic conditions. The study also exhibits that delays and queue lengths are more due to the closely

location of bus stops and on street parking for three wheeled vehicles, thus reducing the effective width of road after the intersections. It is also noticed form the volume data that two – wheelers constitute for highest mode share at all the four intersections. This evident that large portion of commuter's uses personalized mode share than public transport facility. Delay and queue lengths are reduced with the provision of alternative directional route for 2-wheelers, 'U' turn priority ahead of intersection, through lane prioritization and regulation in left turn traffic movements at the study intersections. The Level of service has a significant improvement

at all study intersections within the available road geometry.

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